

Low Pressure Pipe System Design and Construction

Tall Guy Waste Water Solutions & Soils, LLC

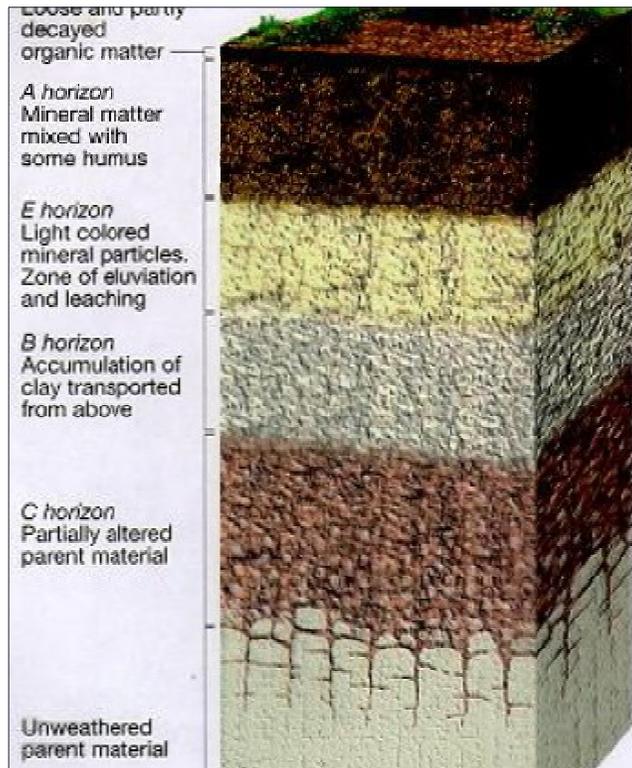
Chris Nothstine, PE

The materials being presented today represent MY opinions, based on MY experiences and do NOT necessarily reflect the opinions of NOWRA.



Soil morphology

Soil morphology is the field of observable attributes of the soil within the various soil horizons and the description of the kind and arrangement of the horizons. The observable attributes ordinarily described in the field include the composition (texture), soil structure and organization of the soil, color of the base soil and features such as mottling, distribution of roots and pores, evidence of translocated materials such as carbonates, iron, manganese, carbon and clay, and the consistence of the soil.





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Onsite Wastewater Treatment

DHSS Home » Healthy Living » Environmental Factors » onsite

- Onsite Wastewater Treatment Systems
- Registration & Licensure Forms
- Information for Professionals
- Calendar of Events
- Onsite Wastewater Complaints
- CEU Tracking Information ***NEW***
- Onsite Wastewater Courses
- Related Links
- Laws, Regulations & Manuals
- Frequently Asked Questions
- DHSS Construction Permit Process and Application

Operation and Maintenance Guidelines

An estimated 25 percent of homes in Missouri rely on an onsite wastewater treatment system (OWTS) in areas where public sewers are not available. Onsite systems treat wastewater and disperse it on the property where it is generated. When functioning properly, onsite systems prevent human contact with sewage, and prevent contamination of surface and groundwater. Factors that affect the proper functioning of onsite systems include the site and soil conditions, design, installation, operation, and maintenance.

The Missouri Department of Health and Senior Services (DHSS) has set minimum state standards for OWTS. These standards cover new systems and major changes to existing systems. Some examples of major changes are replacing a sewage tank, and replacing or expanding an absorption field. These standards became effective in January 1996.



Healthy Living

Environmental Factors

Chronic Diseases

Communicable Diseases

Healthy Families

Organ/Tissue Donation and Registry

Women, Infants & Children (WIC)

Genetic Disease & Early Childhood

Food Programs

Wellness & Prevention

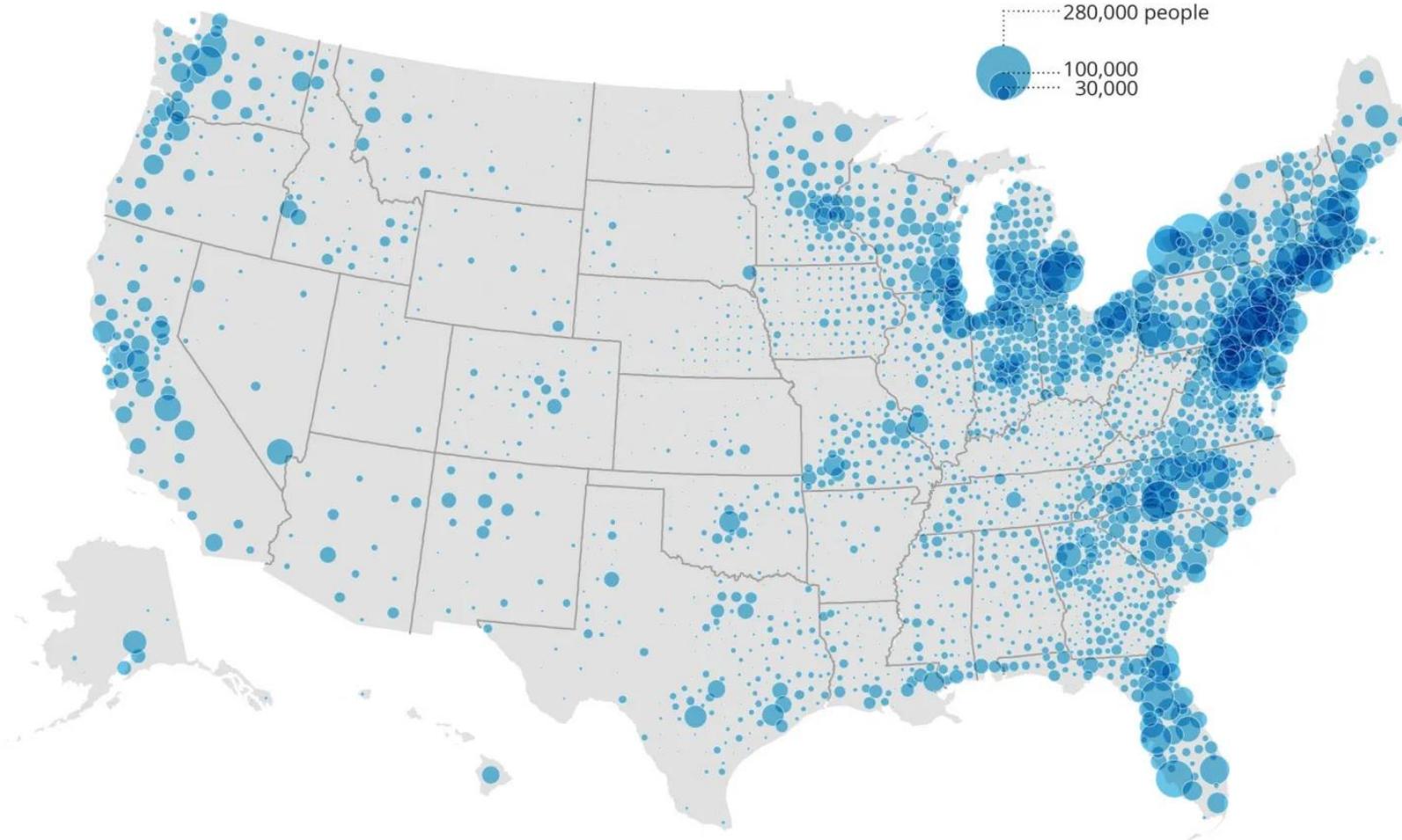
Local Public Health Agencies

Immunizations

from circle of blue WaterNews, October 1, 2018

Number of People in each County who use Household Wells (Domestic, self-supplied population)

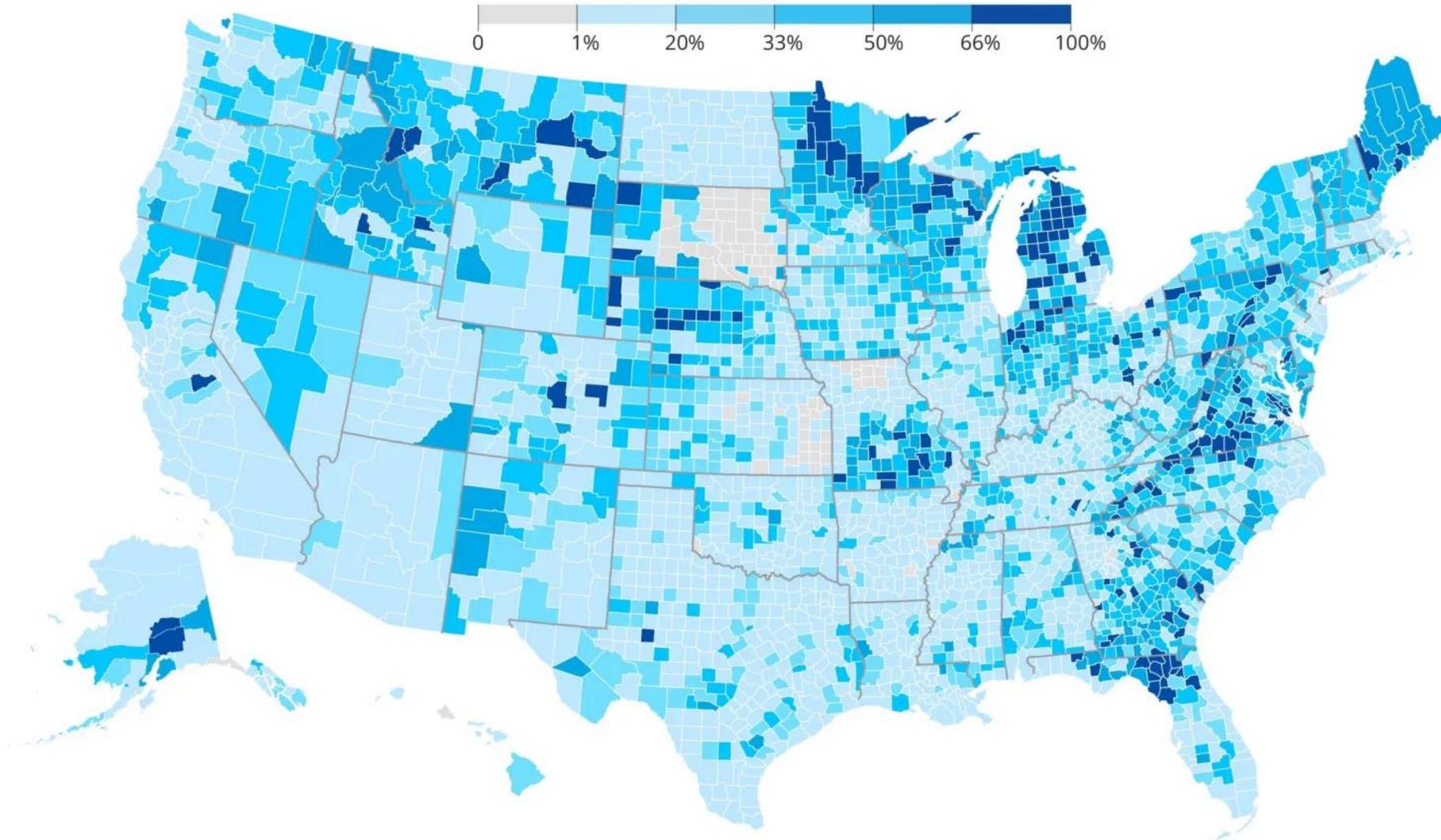
Source: U.S. Geological Survey



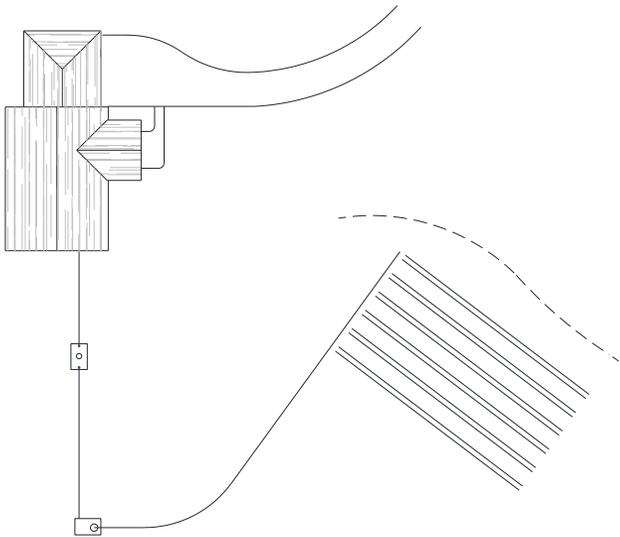
from circle of blue WaterNews, October 1, 2018

Percentage of People in each County who use Household Wells (Domestic, self-supplied population)

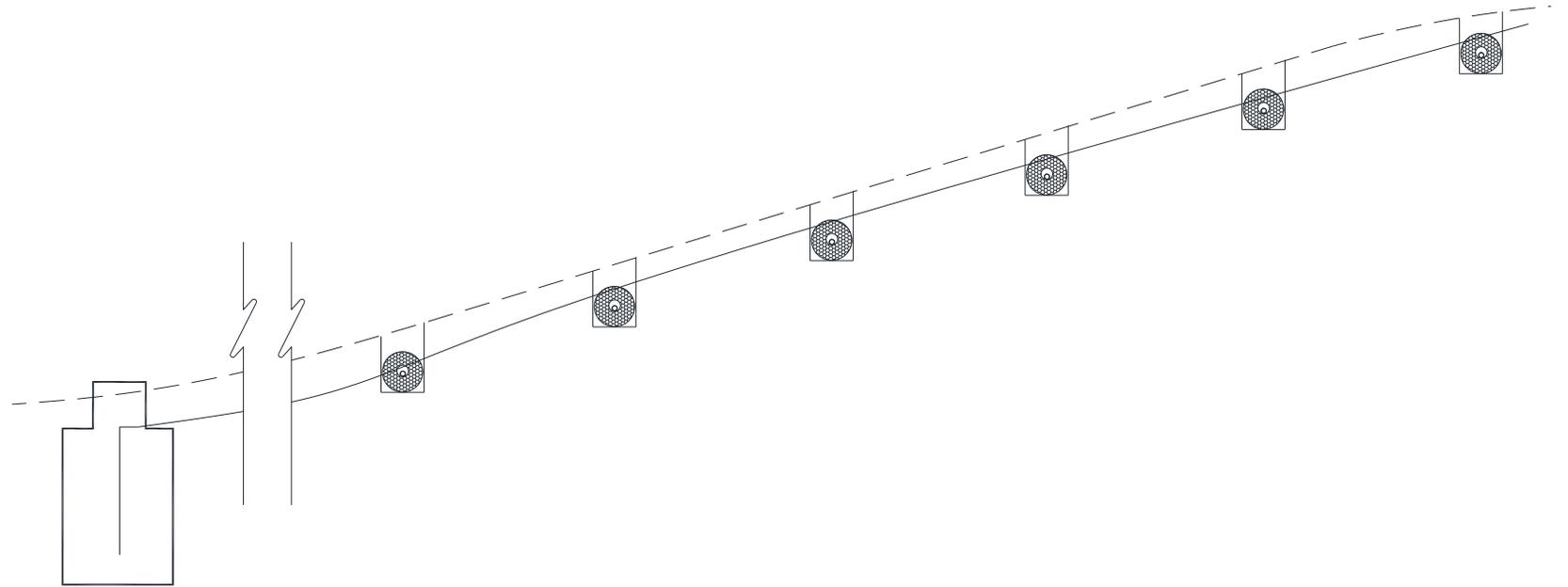
Source: U.S. Geological Survey



LPP design and construction

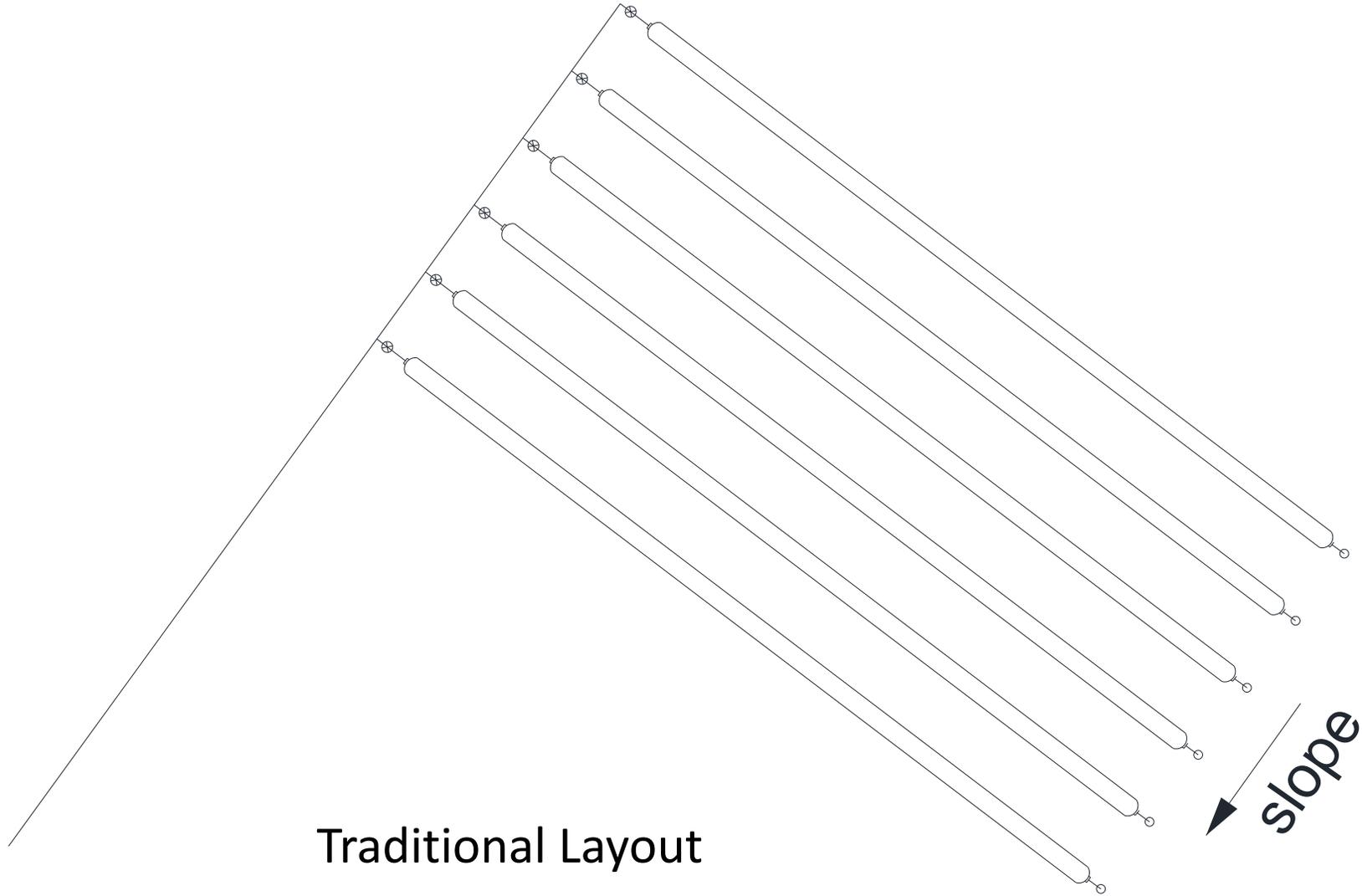


Plan View



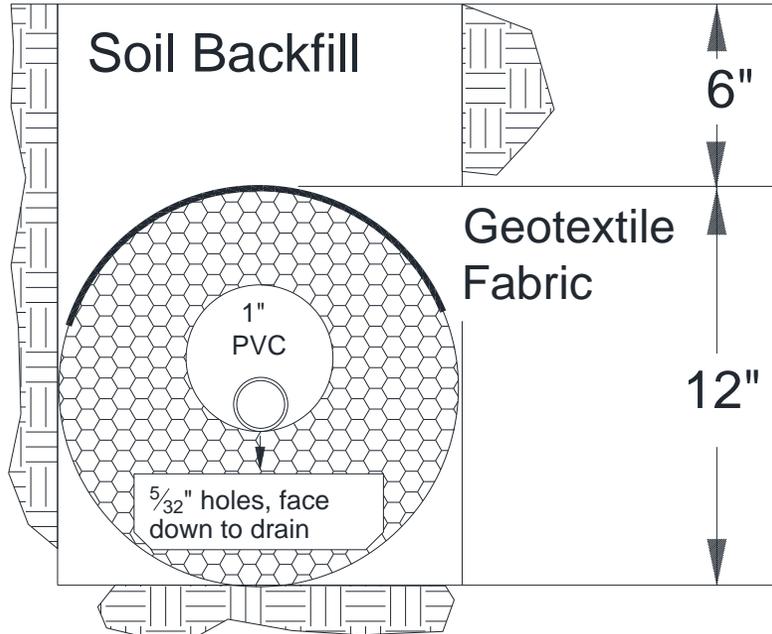
Profile of tank and laterals

LPP design and construction

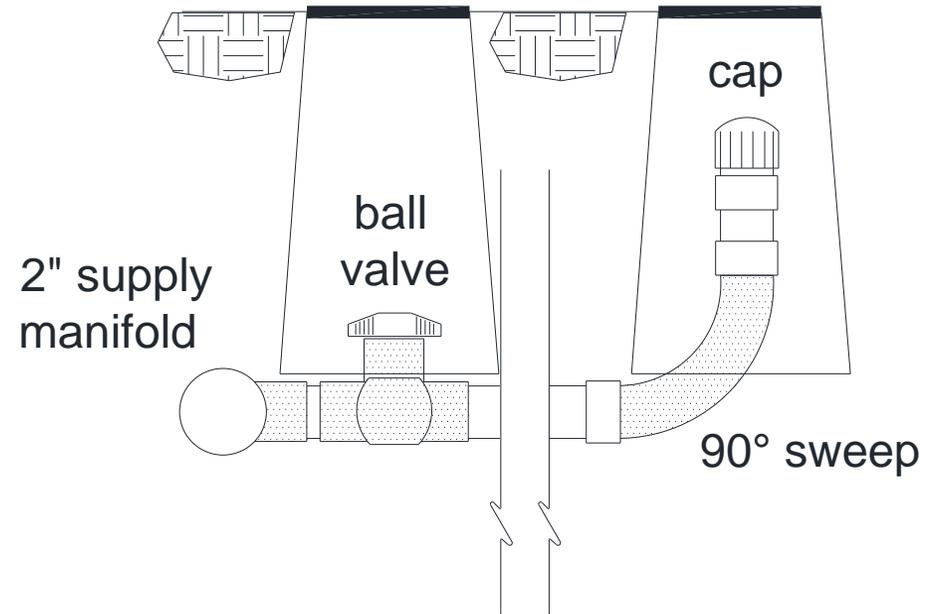


LPP design and construction

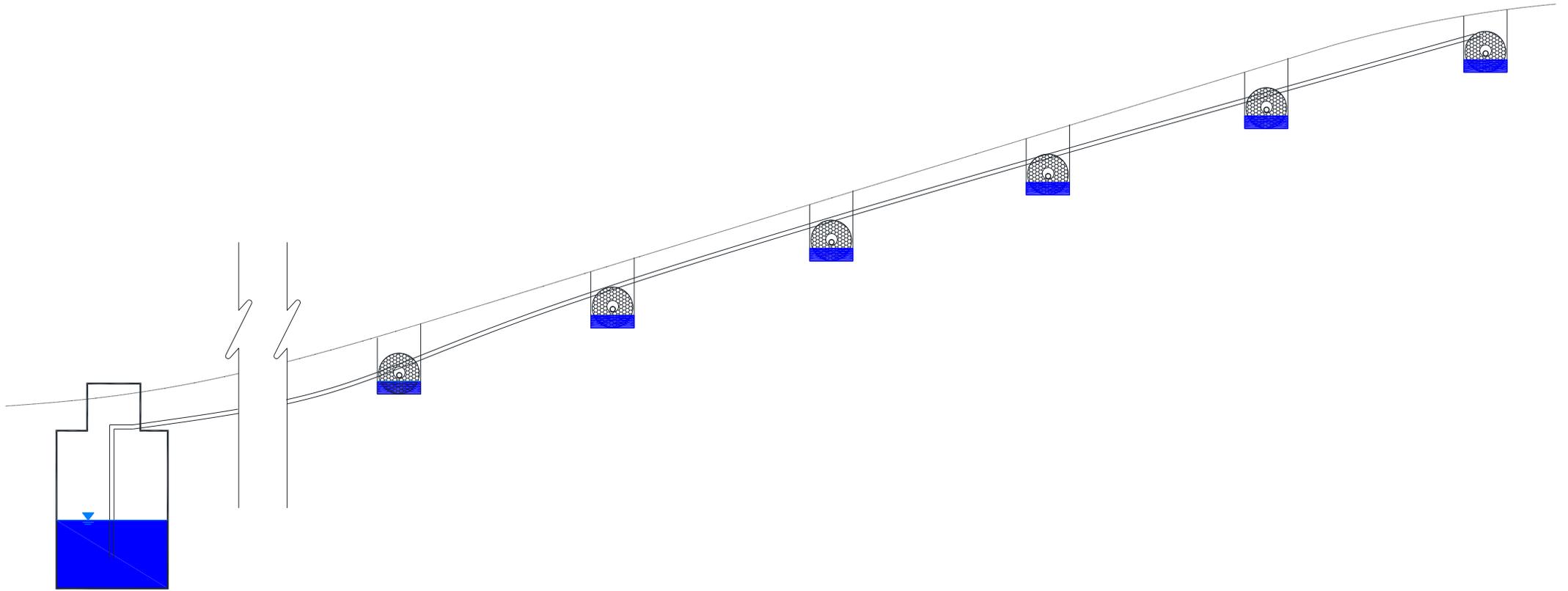
Typical Lateral (NTS)



Lateral Profile (NTS)

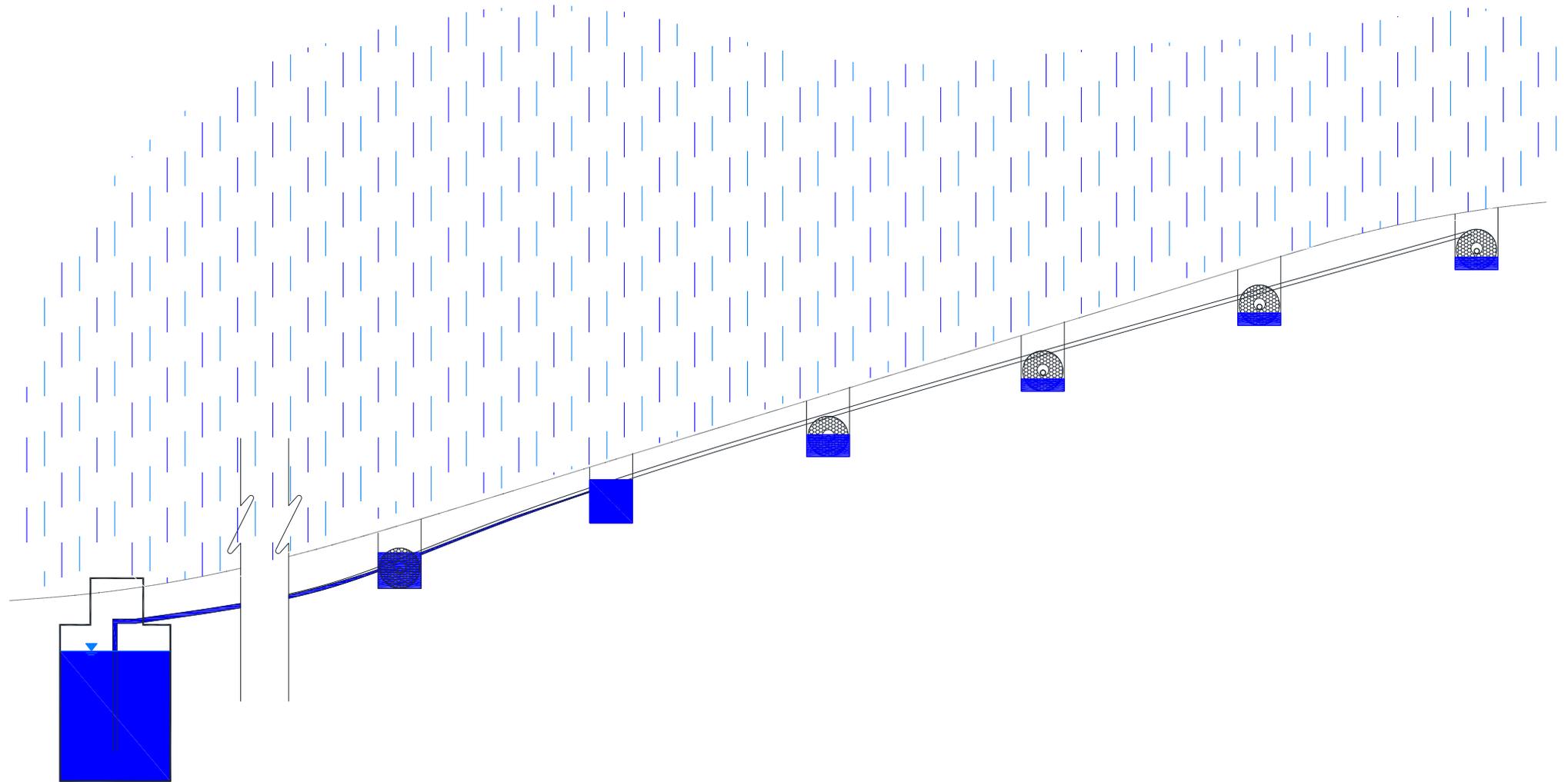


LPP design and construction



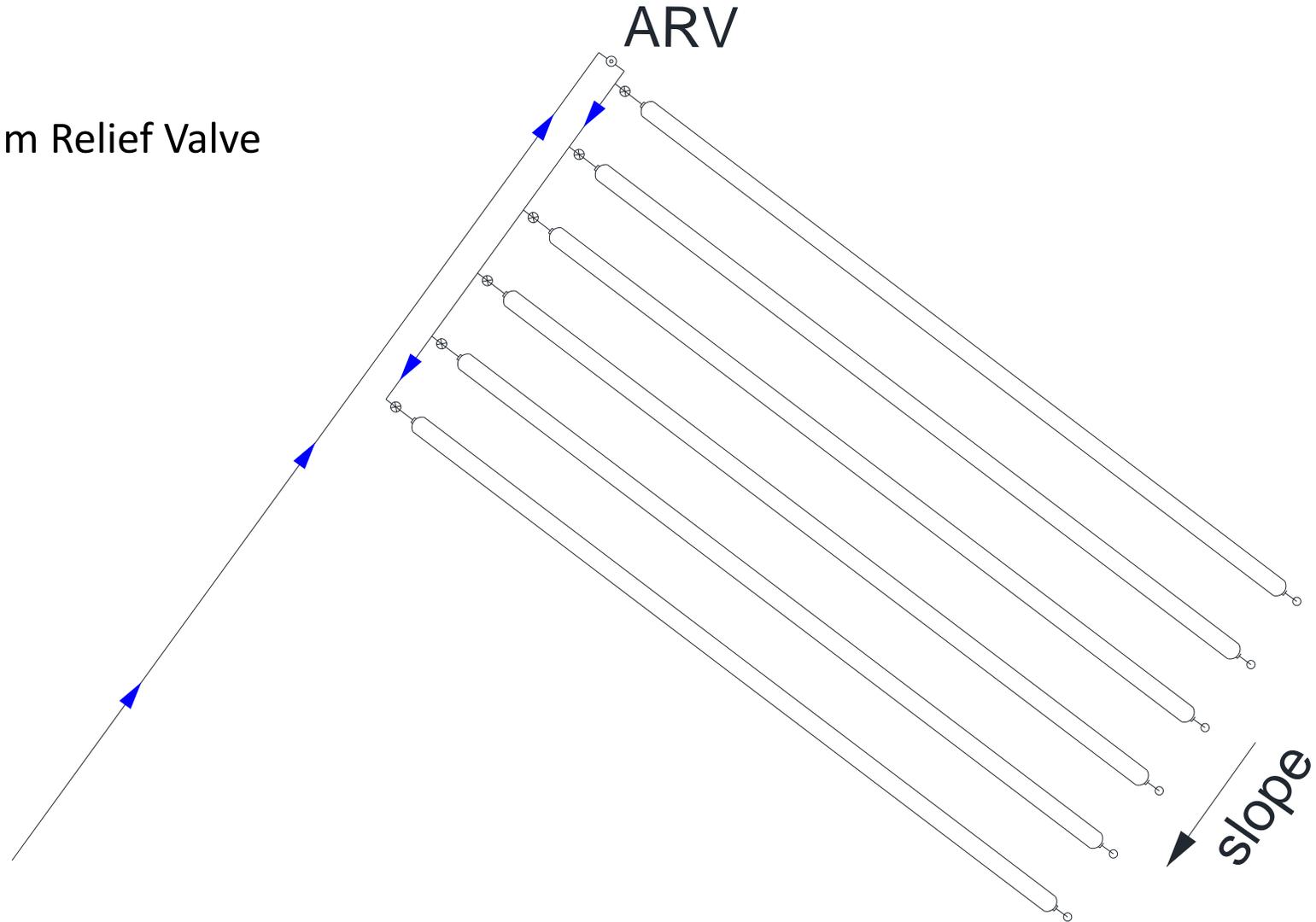
Traditional Layout

LPP design and construction

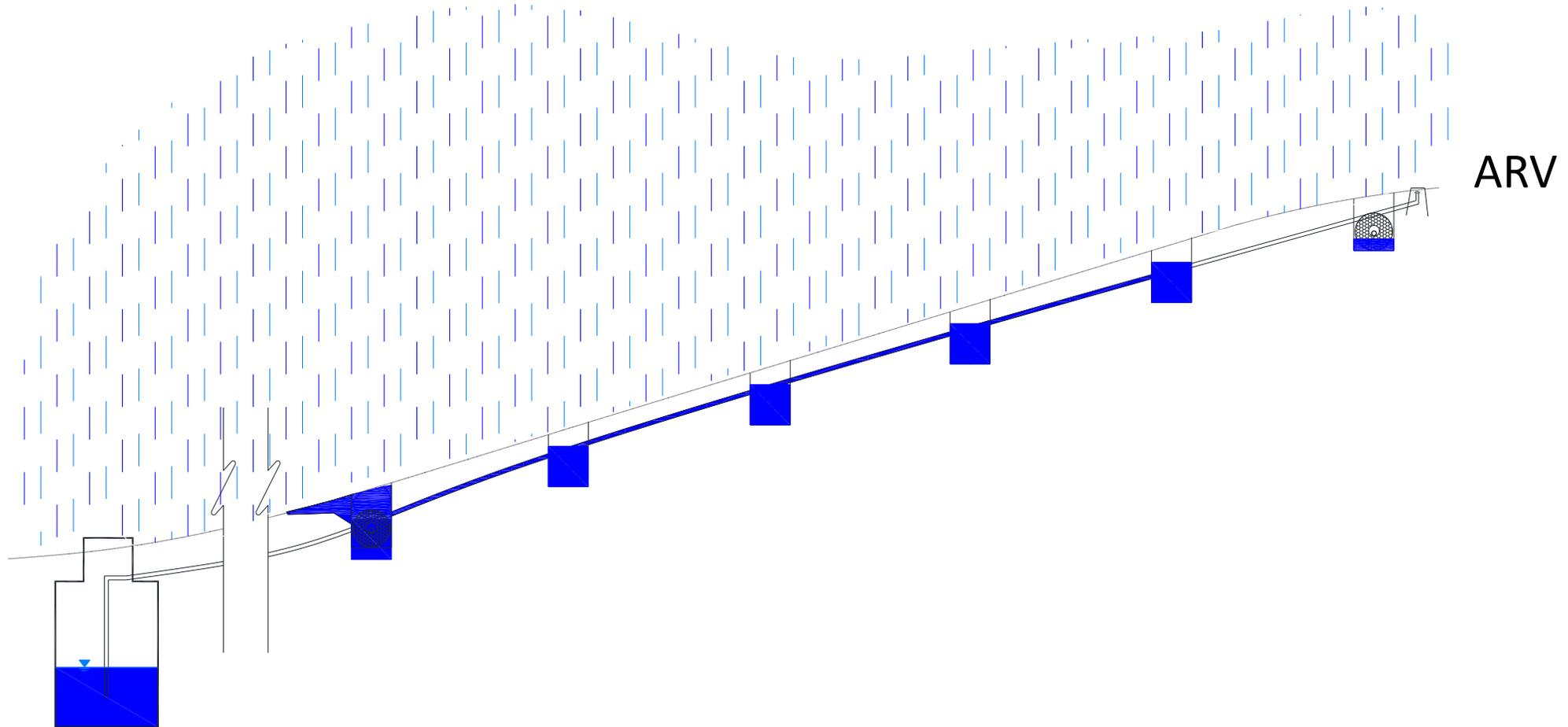


LPP design and construction

ARV=Air/Vacuum Relief Valve

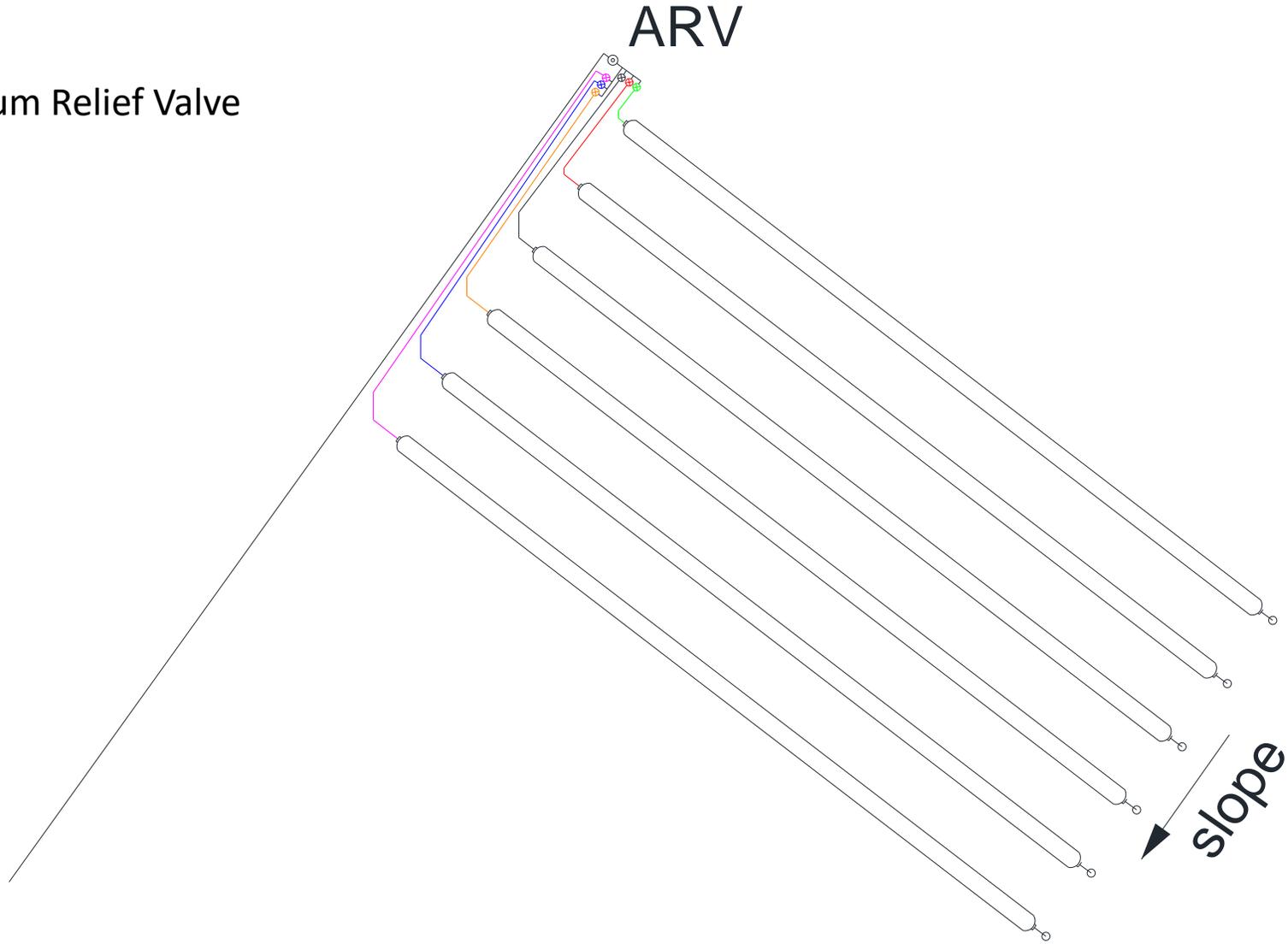


LPP design and construction

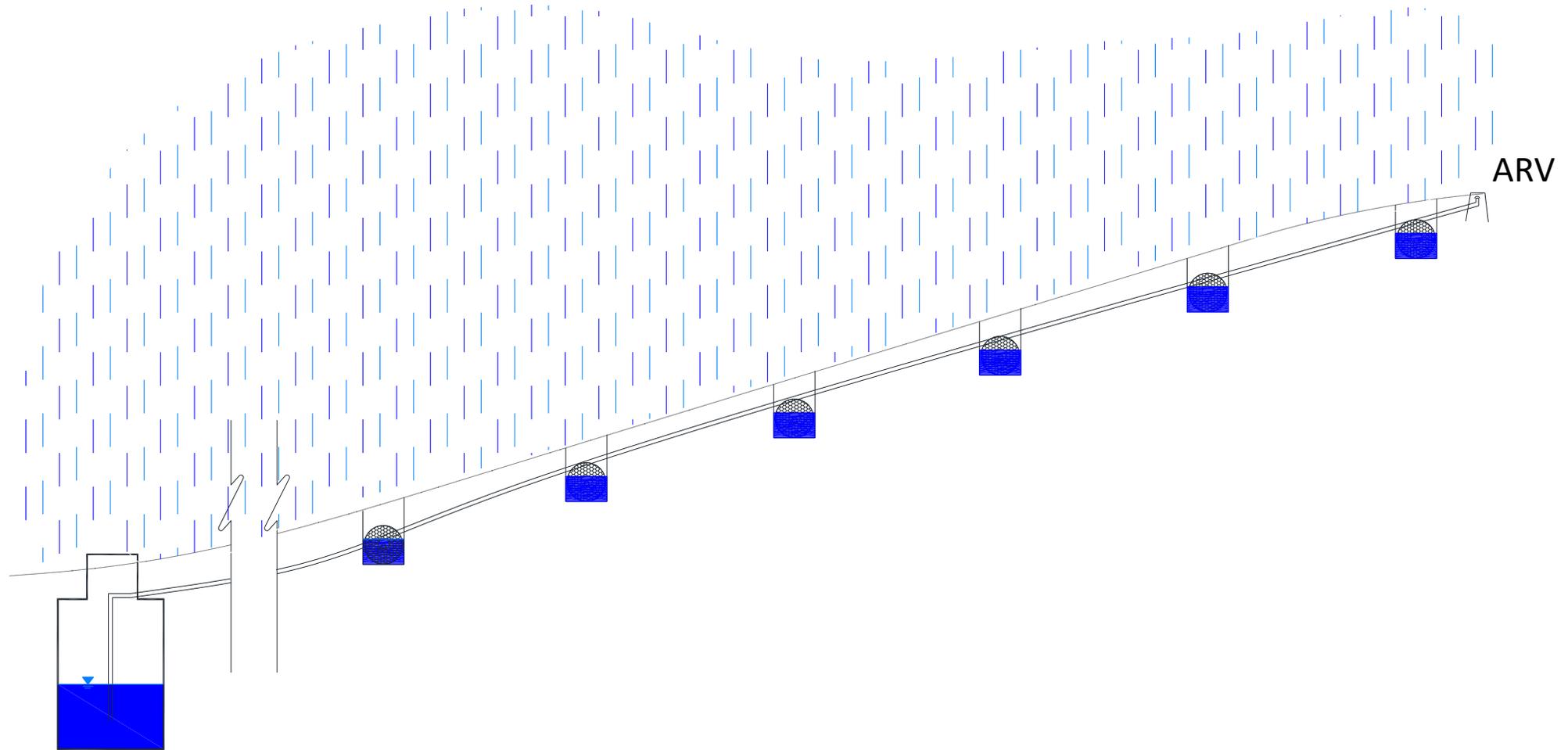


LPP design and construction

ARV=Air/Vacuum Relief Valve



LPP design and construction



LPP design and construction

SIZING INFORMATION

TABLE 4: DISCHARGE RATES IN GALLONS PER MINUTE FROM ORIFICES¹

Pressure (ft)	Orifice Diameter (in)			
	1/8	5/32	3/16	1/4
2.5	NP	NP	0.66	1.17
3	NP	NP	0.72	1.28
3.5	NP	0.54	0.78	1.38
4	NP	0.58	0.83	1.47
4.5	NP	0.61	0.88	1.56
5	0.41	0.64	0.93	1.65
5.5	0.43	0.68	0.97	1.73
6	0.45	0.71	1.02	1.80
6.5	0.47	0.73	1.06	1.88
7	0.49	0.76	1.10	1.95
7.5	0.50	0.79	1.14	2.02
8	0.52	0.81	1.17	2.08
8.5	0.54	0.84	1.21	2.15
9	0.55	0.86	1.24	2.21
9.5	0.57	0.89	1.28	2.27
10	0.58	0.91	1.31	2.33

NOTE: ¹Table is based on discharge in GPM = 11.79 x Orifice Diameter² in Inches x (Pressure in Feet)^{1/2}. NP means Not Permitted.

Source: *Pressure Distribution Network Design* by James C. Converse, January 2000.

Prepared for you by 10/19/2022
Pump Selection for Pressurized System

Orifice Size	0.156	inches	
Residual Head at Last Orifice	4.00	feet	
Lateral Length	60	feet	
Total Number of Laterals per Cell	6		
Orifice Spacing	5.00	feet	
Distributing Valve Model (# of Zones)	1		None used
Lift to Manifold	13	feet	
Discharge Assembly Size	1.50	inches	
Transport Line Size	1.50	inches	
Pipe Class/Schedule	40		
Transport Length	130	feet	
Manifold Size	2.00	inches	
Pipe Class/Schedule	40		
Length of Distribution Header	5.00	feet	
Lateral Size	1.25	inches	
Pipe Class/Schedule	40		
Flow Meter	none		None used
'Add-on' Friction Losses	5.00	feet	

Input

Calculation	
Minimum Flow Rate per Orifice	0.60 gpm
Number of Orifices per Zone	72
Total Actual Flow Rate	43.6 gpm
Number of Laterals per Zone	6
Total Dynamic Head:	
Lift to Manifold	13.0 feet
Residual Head at Last Orifice	4.0 feet

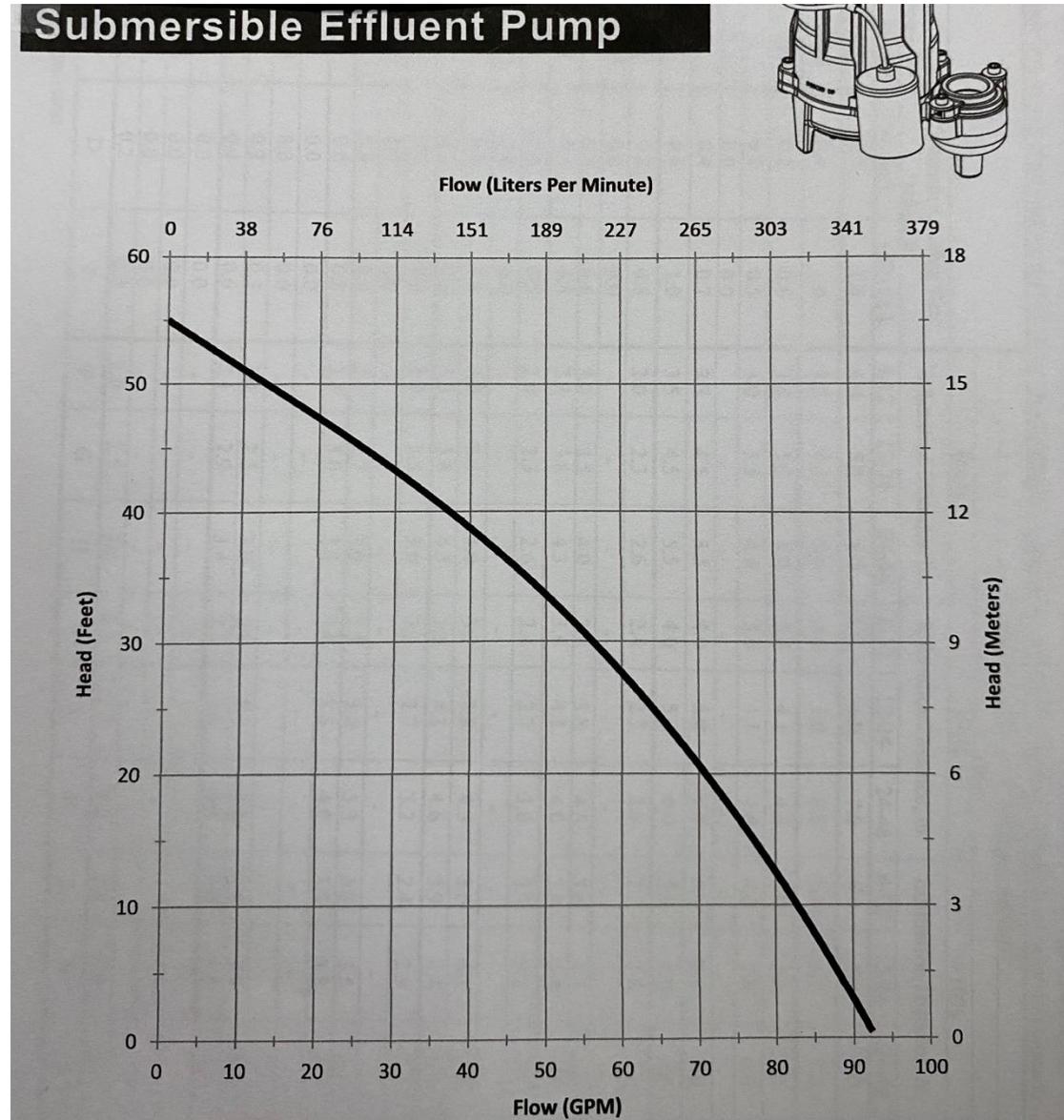
Frictional Head Losses:	
Head Loss in Transport Pipe	13.7 feet
Head Loss through Discharge Assembly	5.7 feet
Head Loss in Distribution Header	0.0 feet
Head Loss in Laterals	0.2 feet
'Add-on' Friction Losses	5.0 feet
Head Loss through Distributing Valve	0.0 feet None Used
Head Loss through Flow Meter	0.0 feet None Used

Size Pump for:

TOTAL FLOW RATE	43.6 gpm
	@
TOTAL DYNAMIC HEAD	41.7 feet

Output

LPP design and construction



Prepared for you by

10/19/2022

Pump Selection for Pressurized System

Orifice Size	0.156	inches	
Residual Head at Last Orifice	4.00	feet	
Lateral Length	60	feet	
Total Number of Laterals per Cell	6		
Orifice Spacing	5.00	feet	
Distributing Valve Model (# of Zones)	1		None used
Lift to Manifold	13	feet	
Discharge Assembly Size	1.50	inches	
Transport Line Size	2.00	inches	
Pipe Class/Schedule	40		
Transport Length	130	feet	
Manifold Size	2.00	inches	
Pipe Class/Schedule	40		
Length of Distribution Header	5.00	feet	
Lateral Size	1.25	inches	
Pipe Class/Schedule	40		
Flow Meter	none		None used
'Add-on' Friction Losses	5.00	feet	

Calculation	
Minimum Flow Rate per Orifice	0.60 gpm
Number of Orifices per Zone	72
Total Actual Flow Rate	43.6 gpm
Number of Laterals per Zone	6
Total Dynamic Head:	
Lift to Manifold	13.0 feet
Residual Head at Last Orifice	4.0 feet
Frictional Head Losses:	
Head Loss in Transport Pipe	4.1 feet
Head Loss through Discharge Assembly	5.7 feet
Head Loss in Distribution Header	0.0 feet
Head Loss in Laterals	0.2 feet
'Add-on' Friction Losses	5.0 feet
Head Loss through Distributing Valve	0.0 feet None Used
Head Loss through Flow Meter	0.0 feet None Used
Size Pump for:	
TOTAL FLOW RATE	43.6 gpm
	@
TOTAL DYNAMIC HEAD	32.0 feet

LPP design and construction

When designing low-pressure pipe systems, we need to consider Linear Loading Rate.

LLR – Amount of wastewater applied daily along the landscape contour. It is expressed in gallons per day per lineal foot along the contour.

J.C. Converse, 1998

LPP design and construction

Depth of Class III soil (inches)	Maximum recommended LLR
18-24	4 gpd/LF
12-18	3 gpd/LF
6-12	1 gpd/LF

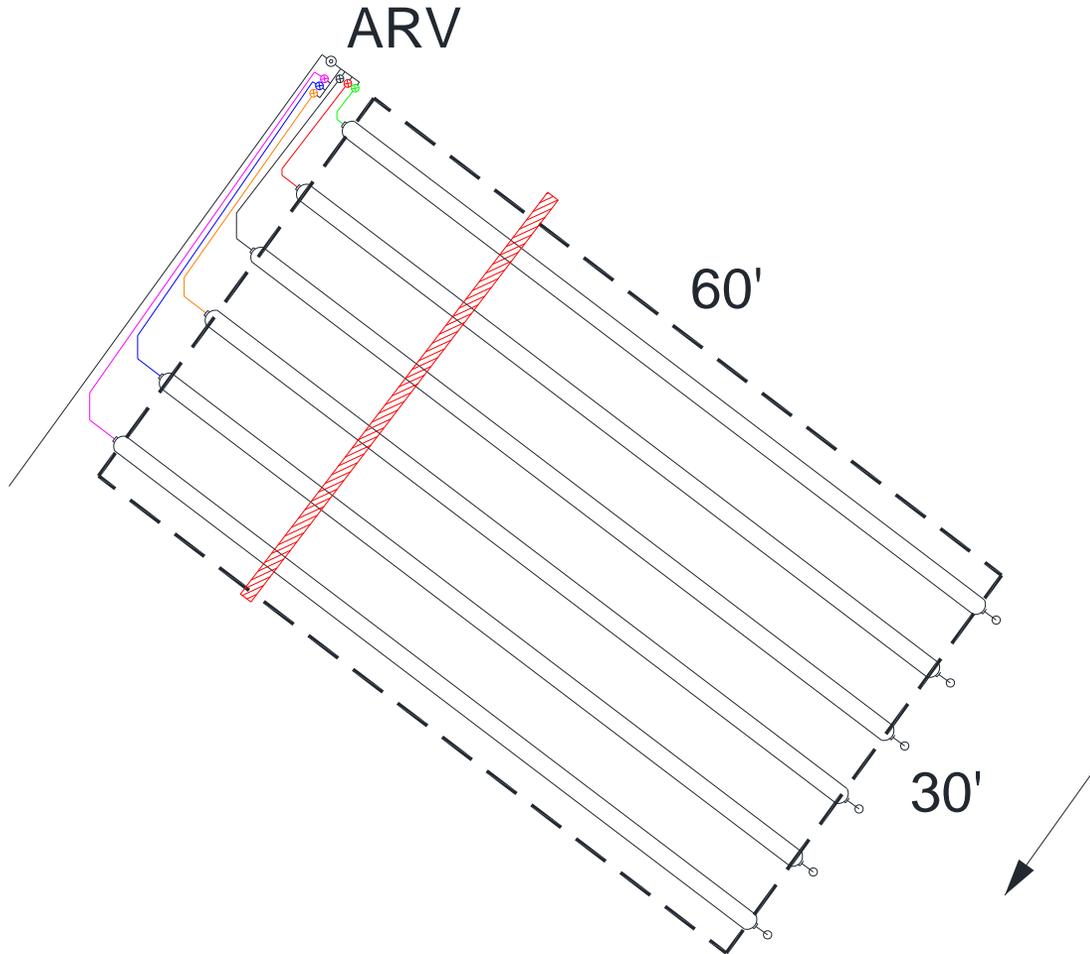
J.C. Converse

LPP design and construction

Table 1. Infiltration rates in gal/da/ft² for wastewater of >30 mg L⁻¹ or wastewater of <30 mg L⁻¹ and hydraulic linear loading rates in gal/da/ft for soil characteristics of texture and structure and site conditions of slope and infiltration distance. Values assume wastewater volume of >150 gal/da/bedroom. If horizon consistence is stronger than firm or any cemented class or the clay mineralogy is smectitic, the horizon is limiting regardless of other soil characteristics

Soil Characteristics					Hydraulic Linear Loading Rate, gal/da/ft										Row
					Slope										
					0-4%			5-9%			>10%				
Texture	Structure		Infiltration Loading Rate, gal/da/ft ²		Infiltration Distance, in.			Infiltration Distance, in.			Infiltration Distance, in.				
	Shape	Grade	>30 mg/L	<30 mg/L	8-12	12-24	24-48	8-12	12-24	24-48	8-12	12-24	24-48		
COS, S, LCOS, LS	--	OSG	0.8	1.6	4.0	5.0	6.0	5.0	6.0	7.0	6.0	7.0	8.0	1	
FS, VFS,LFS,LVFS	--	OSG	0.4	1.0	3.5	4.5	5.5	4.0	5.0	6.0	5.0	6.0	7.0	2	
CSL, SL	--	OM	0.2	0.6	3.0	3.5	4.0	3.6	4.1	4.6	5.0	6.0	7.0	3	
	PL	1	0.2	0.5	3.0	3.5	4.0	3.6	4.1	4.6	4.0	5.0	6.0	4	
		2, 3	0.0	0.0	-	-	-	-	-	-	-	-	-	5	
	PR/BK /GR	1	0.4	0.7	3.5	4.5	5.5	4.0	5.0	6.0	5.0	6.0	7.0	6	
		2,3	0.6	1.0	3.5	4.5	5.5	4.0	5.0	6.0	5.0	6.0	7.0	7	
FSL, VFSL	--	OM	0.2	0.5	2.0	2.3	2.6	2.4	2.7	3.0	2.7	3.2	3.7	8	
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-	9	
	PR/BK /GR	1	0.2	0.6	3.0	3.5	4.0	3.3	3.8	4.3	3.6	4.1	4.6	10	
		2,3	0.4	0.8	3.3	3.8	4.3	3.6	4.1	4.6	3.9	4.4	4.9	11	
L	--	OM	0.2	0.5	2.0	2.3	2.6	2.4	2.7	3.0	2.7	3.2	3.7	12	
	PL	1,2, 3	0.0	0.0	-	-	-	-	-	-	-	-	-	13	
	PR/BK /GR	1	0.4	0.6	3.0	3.5	4.0	3.3	3.8	4.3	3.6	4.1	4.6	14	
		2, 3	0.6	0.8	3.3	3.8	4.3	3.6	4.1	4.6	3.9	4.4	4.9	15	
SIL	--	OM	0.0	0.2	2.0	2.5	3.0	2.2	2.7	3.2	2.4	2.9	3.4	16	
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-	17	
	PR/BK /GR	1	0.4	0.6	2.4	2.7	3.0	2.7	3.0	3.3	3.0	3.5	4.0	18	
		2,3	0.6	0.8	2.7	3.0	3.3	3.0	3.5	4.0	3.3	3.8	4.3	19	
SCL,CL SICL	--	OM	0.0	0.0	-	-	-	-	-	-	-	-	-	20	
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-	21	
	PR/BK /GR	1	0.2	0.3	2.0	2.5	3.0	2.2	2.7	3.2	2.4	2.9	3.4	22	
		2,3	0.4	0.6	2.4	2.9	3.4	2.7	3.0	3.3	3.0	3.5	4.0	23	
SC, C, SIC	--	OM	0.0	0.0	-	-	-	-	-	-	-	-	-	24	
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-	25	
	PR/BK /GR	1	0.0	0.0	-	-	-	-	-	-	-	-	-	26	
		2,3	0.2	0.3	2.0	2.5	3.0	2.2	2.7	3.2	2.4	2.9	3.4	27	
A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	

LPP design and construction



Area Requirement

$$1,800 \text{ SF} \times 0.2 \text{ gpd/SF} = 360 \text{ gpd}$$

Linear Loading Rate (LLR)

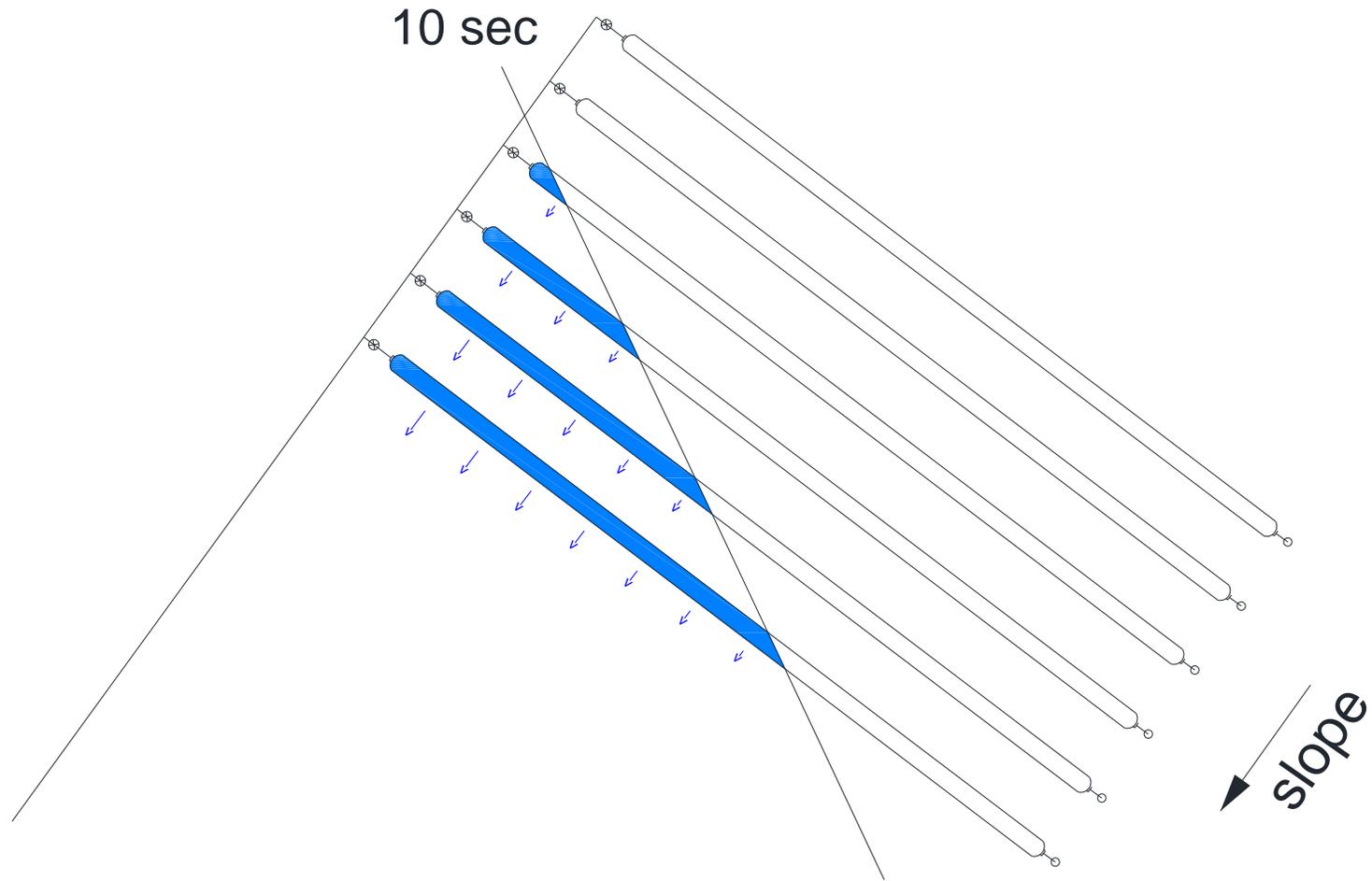
$$60 \text{ gpd} / 60\text{LF} = 1.0 \text{ gpd/LF}$$

(LLR)

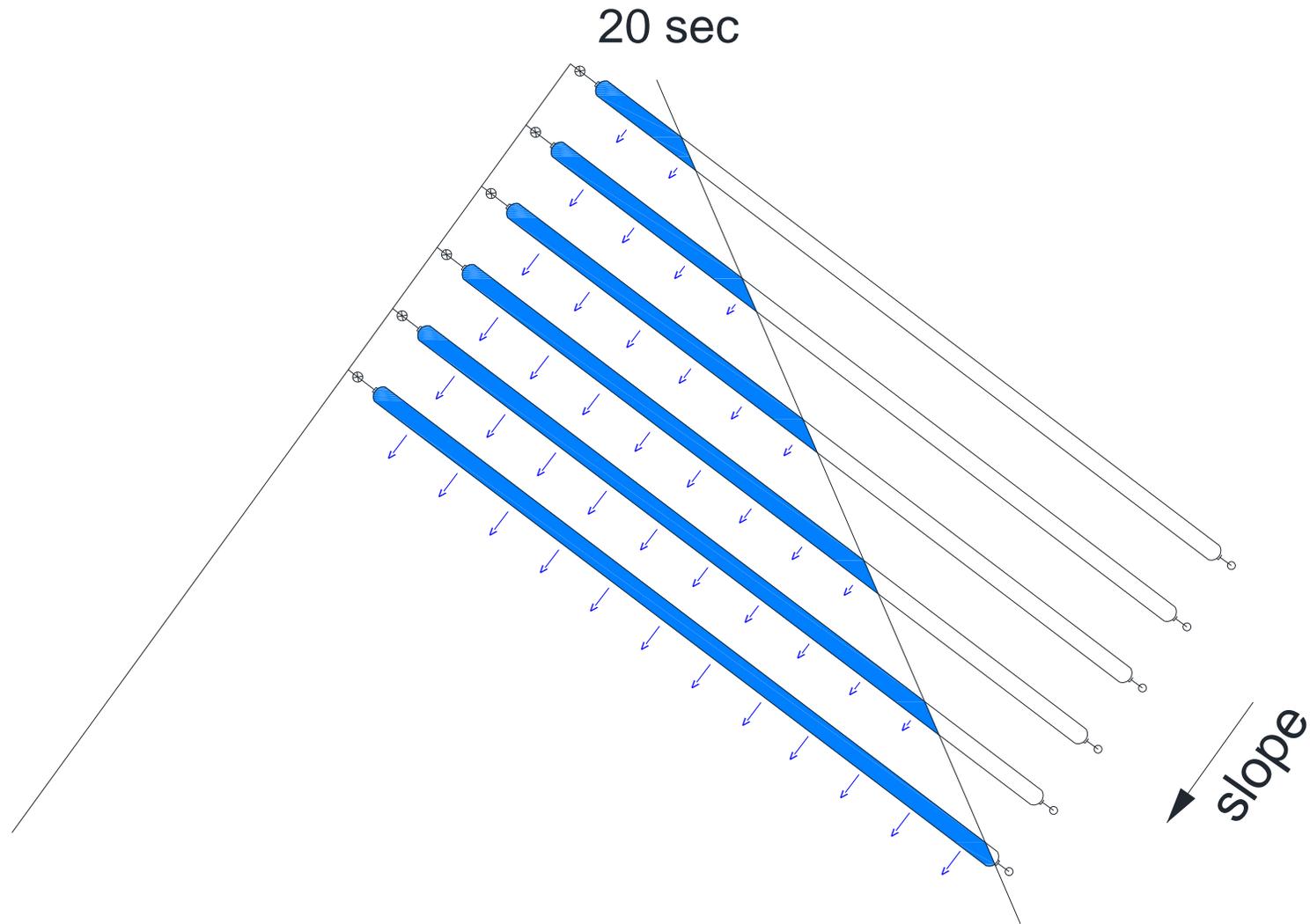
$$1 \text{ gpd/LF} \times 6 = 6 \text{ gpd/LF}$$

$$6 \text{ gpd/LF} > 4 \text{ gpd/LF} \text{ (not advisable)}$$

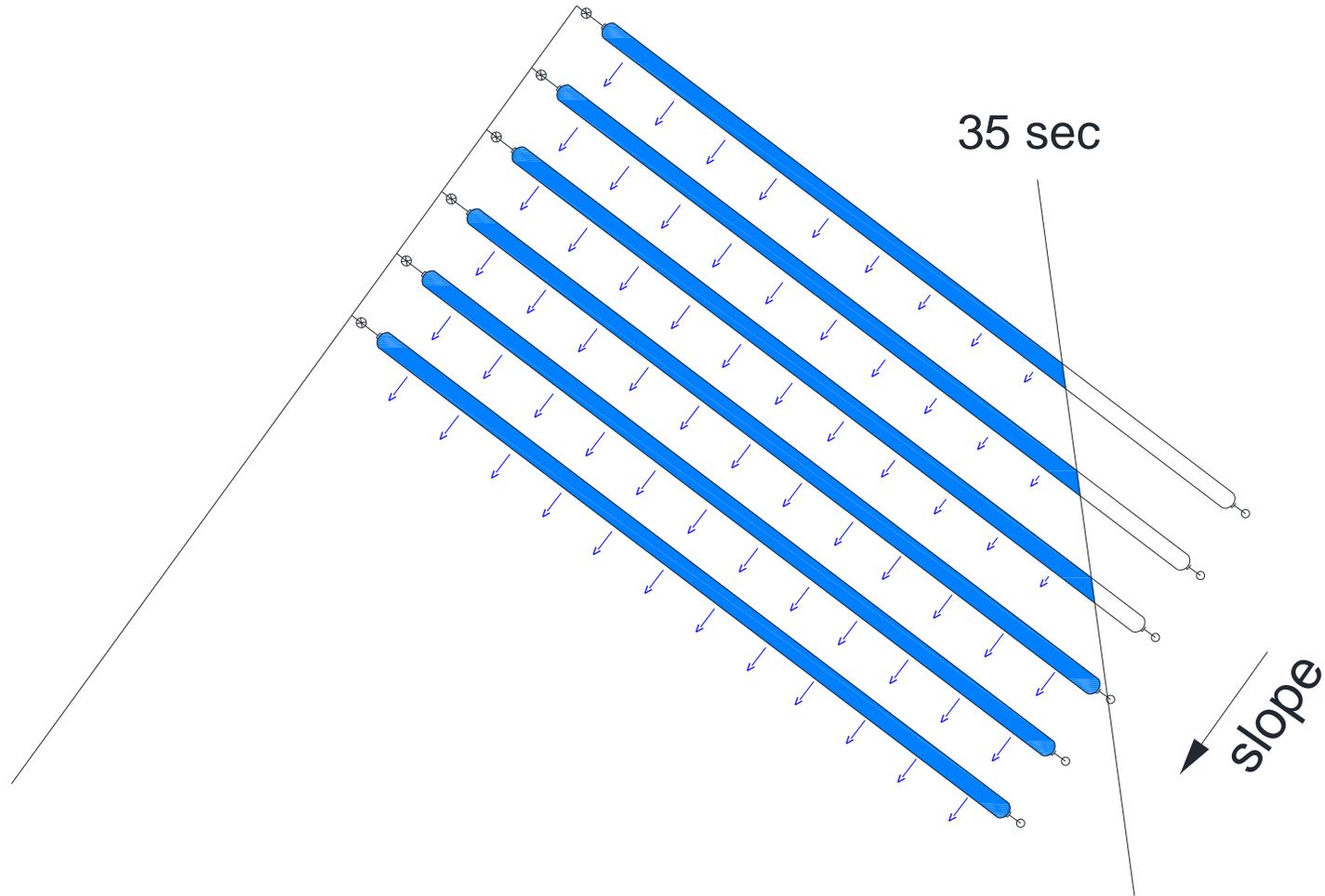
LPP design and construction



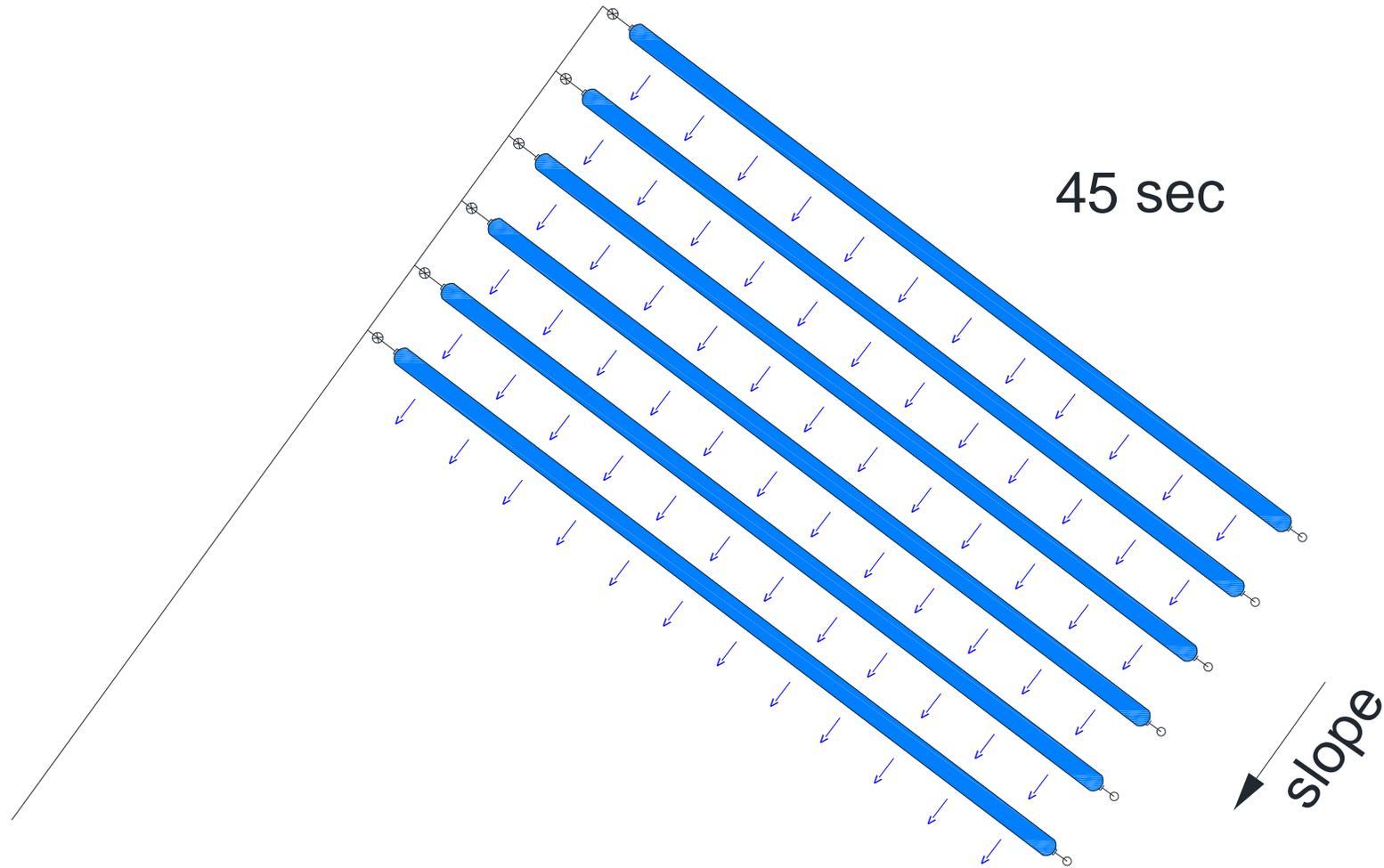
LPP design and construction



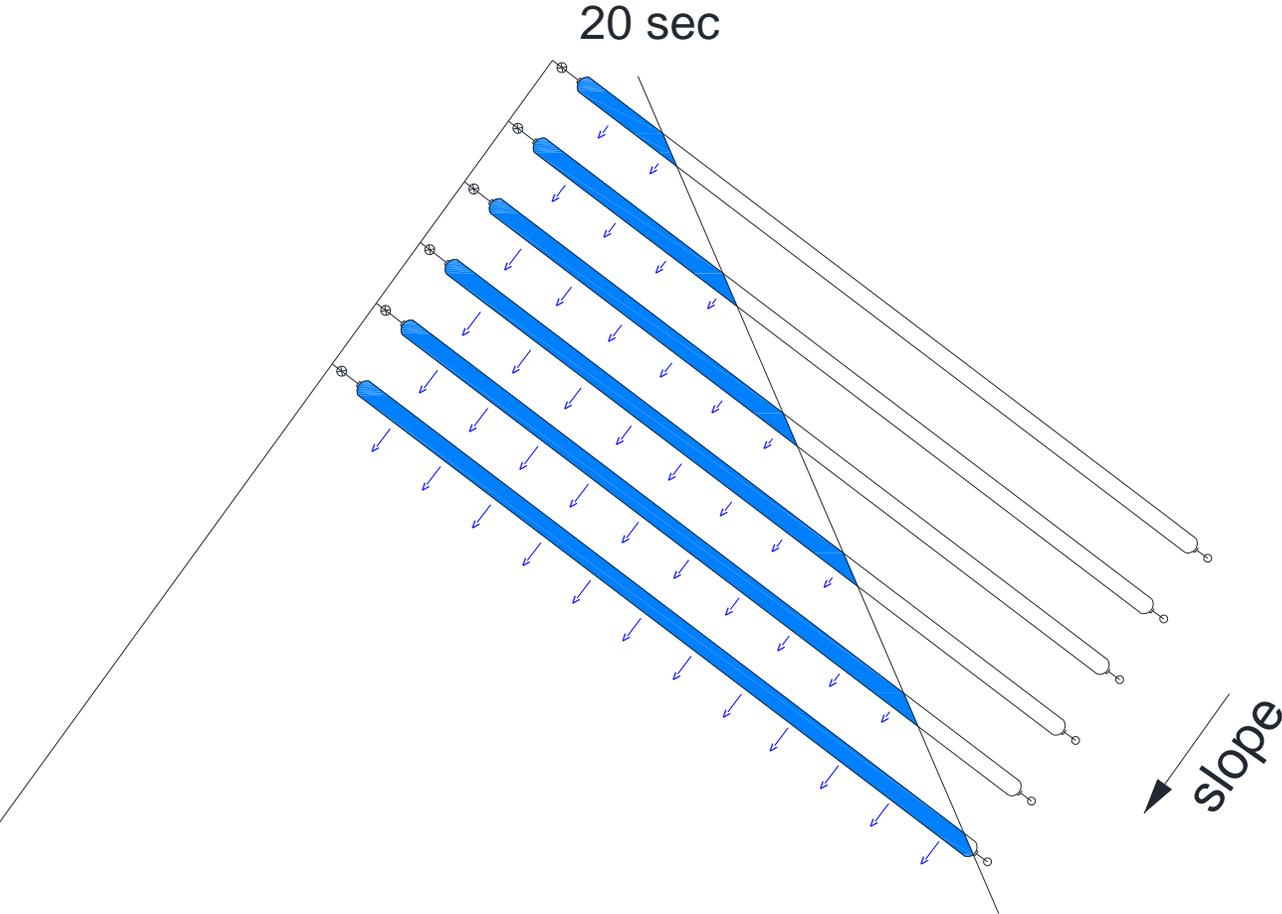
LPP design and construction



LPP design and construction



LPP design and construction



Q(gallons) @ time(minutes)

Line	1.0 min	2.0 min	3.0 min
1	2.0	8.7	15.4
2	2.3	9.0	15.7
3	2.8	9.5	16.2
4	3.6	10.3	17.0
5	4.6	11.3	18.0
6	5.8	12.5	19.2
Total	21.1	61.3	101.5
$\frac{Q_6 - Q_1}{Q_1}$	190%	44%	25%

to the SHWT or to impervious layer.

4. Cover: The cover from the top of the LPP to the proposed grade shall be not less than six inches (6") to eighteen inches (18") (see § 6.84 and 6.85 of this Part, Fig. 6.84 and 6.85).

5. All LPPs shall be timed-dosed, either by the timed-dosed technology preceding (Category 1 technology) or by incorporating a timed-dosed component as specified in § 6.36(B) of this Part.

6. Flow differential between first (1st) and last orifice in the laterals: The maximum head differential between the first (1st) and last orifice on each lateral shall be no greater than fifteen percent (15%).

7. Pump events per day and maximum dose per LPP orifice: The number of dose events per day shall be between twelve (12) and twenty-four (24). The maximum dose per LPP orifice shall be one half (0.50) gallons.

Components of the LPP: LPPs shall also conform to other components in § 6.36(B) of this Part: "Common components for all pressurized drainfields."

40 PVC or equivalent sweep elbows (also called "turnups") shall be attached to each lateral to facilitate maintenance and inspection. A stopcock shall be provided on each lateral.

LPP design and construction

LPP design and construction

ARV=Air/Vacuum Relief Valve

